

Chapter 2

Fundamentals of Flow in Closed Conduits

2.1. From the given data: $D_1 = 0.1$ m, $D_2 = 0.15$ m, $V_1 = 2$ m/s. Using these data, the following preliminary calculations are useful:

$$A_1 = \frac{\pi}{4}D_1^2 = \frac{\pi}{4}(0.1)^2 = 0.007854 \text{ m}^2, \quad A_2 = \frac{\pi}{4}D_2^2 = \frac{\pi}{4}(0.15)^2 = 0.01767 \text{ m}^2$$

Volumetric flow rate, Q , is given by

$$Q = A_1V_1 = (0.007854)(2) = \boxed{0.0157 \text{ m}^3/\text{s}}$$

According to the continuity equation,

$$A_1V_1 = A_2V_2 = Q \quad \rightarrow \quad V_2 = \frac{Q}{A_2} = \frac{0.0157}{0.01767} = \boxed{0.889 \text{ m/s}}$$

At 20°C, the density of water, ρ , is 998 kg/m³, and the mass flow rate, \dot{m} , is given by

$$\dot{m} = \rho Q = (998)(0.0157) = \boxed{15.7 \text{ kg/s}}$$

2.xx From the given data: $D_1 = 0.2$ m, $D_2 = 0.3$ m, and $V_1 = 0.75$ m/s. Using these data, the following preliminary calculations are useful:

$$A_1 = \frac{\pi}{4}D_1^2 = \frac{\pi}{4}(0.2)^2 = 0.03142 \text{ m}^2, \quad A_2 = \frac{\pi}{4}D_2^2 = \frac{\pi}{4}(0.3)^2 = 0.07069 \text{ m}^2$$

(a) According to the continuity equation,

$$A_1V_1 = A_2V_2 \quad \rightarrow \quad V_2 = \frac{A_1V_1}{A_2} = \frac{(0.03142)(0.75)}{0.07069} = \boxed{0.333 \text{ m/s}}$$

(b) The volume flow rate, Q , is given by

$$Q = A_1V_1 = (0.03142)(0.75) = 0.02357 \text{ m}^3/\text{s} = \boxed{23.6 \text{ L/s}}$$

2.2. From the given data: $D_1 = 200$ mm, $D_2 = 100$ mm, $V_1 = 1$ m/s, and

$$A_1 = \frac{\pi}{4}D_1^2 = \frac{\pi}{4}(0.2)^2 = 0.0314 \text{ m}^2$$

$$A_2 = \frac{\pi}{4}D_2^2 = \frac{\pi}{4}(0.1)^2 = 0.00785 \text{ m}^2$$

The flow rate, Q_1 , in the 200-mm pipe is given by

$$Q_1 = A_1V_1 = (0.0314)(1) = 0.0314 \text{ m}^3/\text{s}$$

and hence the flow rate, Q_2 , in the 100-mm pipe is

$$Q_2 = \frac{Q_1}{2} = \frac{0.0314}{2} = \boxed{0.0157 \text{ m}^3/\text{s}}$$

The average velocity, V_2 , in the 100-mm pipe is

$$V_2 = \frac{Q_2}{A_2} = \frac{0.0157}{0.00785} = \boxed{2 \text{ m/s}}$$

2.3. The velocity distribution in the pipe is

$$v(r) = V_0 \left[1 - \left(\frac{r}{R} \right)^2 \right] \quad (1)$$

and the average velocity, \bar{V} , is defined as

$$\bar{V} = \frac{1}{A} \int_A V \, dA \quad (2)$$

where

$$A = \pi R^2 \quad \text{and} \quad dA = 2\pi r \, dr \quad (3)$$

Combining Equations 1 to 3 yields

$$\begin{aligned} \bar{V} &= \frac{1}{\pi R^2} \int_0^R V_0 \left[1 - \left(\frac{r}{R} \right)^2 \right] 2\pi r \, dr = \frac{2V_0}{R^2} \left[\int_0^R r \, dr - \int_0^R \frac{r^3}{R^2} \, dr \right] = \frac{2V_0}{R^2} \left[\frac{R^2}{2} - \frac{R^4}{4R^2} \right] \\ &= \frac{2V_0}{R^2} \frac{R^2}{4} = \boxed{\frac{V_0}{2}} \end{aligned}$$

The flow rate, Q , is therefore given by

$$Q = A\bar{V} = \boxed{\frac{\pi R^2 V_0}{2}}$$

2.4.

$$\begin{aligned} \beta &= \frac{1}{A\bar{V}^2} \int_A v^2 \, dA = \frac{4}{\pi R^2 V_0^2} \int_0^R V_0^2 \left[1 - \frac{2r^2}{R^2} + \frac{r^4}{R^4} \right] 2\pi r \, dr \\ &= \frac{8}{R^2} \left[\int_0^R r \, dr - \int_0^R \frac{2r^3}{R^2} \, dr + \int_0^R \frac{r^5}{R^4} \, dr \right] = \frac{8}{R^2} \left[\frac{R^2}{2} - \frac{R^4}{2R^2} + \frac{R^6}{6R^4} \right] \\ &= \boxed{\frac{4}{3}} \end{aligned}$$

2.5. $D = 0.2$ m, $Q = 0.06$ m³/s, $L = 100$ m, $p_1 = 500$ kPa, $p_2 = 400$ kPa, $\gamma = 9.79$ kN/m³.

$$R = \frac{D}{4} = \frac{0.2}{4} = 0.05 \text{ m}$$

$$\Delta h = \frac{p_1}{\gamma} - \frac{p_2}{\gamma} = \frac{500 - 400}{9.79} = 10.2 \text{ m}$$

$$\tau_0 = \frac{\gamma R \Delta h}{L} = \frac{(9.79 \times 10^3)(0.05)(10.2)}{100} = \boxed{49.9 \text{ N/m}^2}$$

$$A = \frac{\pi D^2}{4} = \frac{\pi(0.2)^2}{4} = 0.0314 \text{ m}^2$$

$$V = \frac{Q}{A} = \frac{0.06}{0.0314} = 1.91 \text{ m/s}$$

$$f = \frac{8\tau_0}{\rho V^2} = \frac{8(49.9)}{(998)(1.91)^2} = \boxed{0.11}$$

2.6. $T = 20^\circ\text{C}$, $V = 2$ m/s, $D = 0.25$ m, horizontal pipe, ductile iron. For ductile iron pipe, $k_s = 0.26$ mm, and

$$\frac{k_s}{D} = \frac{0.26}{250} = 0.00104$$

$$\text{Re} = \frac{\rho V D}{\mu} = \frac{(998.2)(2)(0.25)}{(1.002 \times 10^{-3})} = 4.981 \times 10^5$$

From the Moody diagram:

$$\boxed{f = 0.0202 \text{ (flow is not fully turbulent)}}$$

Using the Colebrook equation,

$$\frac{1}{\sqrt{f}} = -2 \log \left(\frac{k_s/D}{3.7} + \frac{2.51}{\text{Re}\sqrt{f}} \right)$$

Substituting for k_s/D and Re gives

$$\frac{1}{\sqrt{f}} = -2 \log \left(\frac{0.00104}{3.7} + \frac{2.51}{4.981 \times 10^5 \sqrt{f}} \right)$$

By trial and error leads to

$$\boxed{f = 0.0204}$$

Using the Swamee-Jain equation,

$$\frac{1}{\sqrt{f}} = -2 \log \left[\frac{k_s/D}{3.7} + \frac{5.74}{\text{Re}^{0.9}} \right] = -2 \log \left[\frac{0.00104}{3.7} + \frac{5.74}{(4.981 \times 10^5)^{0.9}} \right]$$

which leads to

$$\boxed{f = 0.0205}$$

The head loss, h_f , over 100 m of pipeline is given by

$$h_f = f \frac{L V^2}{D 2g} = 0.0204 \frac{100}{0.25} \frac{(2)^2}{2(9.81)} = 1.66 \text{ m}$$

Therefore the pressure drop, Δp , is given by

$$\Delta p = \gamma h_f = (9.79)(1.66) = \boxed{16.3 \text{ kPa}}$$

If the pipe is 1 m lower at the downstream end, f would not change, but the pressure drop, Δp , would then be given by

$$\Delta p = \gamma(h_f - 1.0) = 9.79(1.66 - 1) = \boxed{6.46 \text{ kPa}}$$

2.7. From the given data: $D = 25 \text{ mm}$, $k_s = 0.1 \text{ mm}$, $\theta = 10^\circ$, $p_1 = 550 \text{ kPa}$, and $L = 100 \text{ m}$. At 20°C , $\nu = 1.00 \times 10^{-6} \text{ m}^2/\text{s}$, $\gamma = 9.79 \text{ kN/m}^3$, and

$$\begin{aligned} \frac{k_s}{D} &= \frac{0.1}{25} = 0.004 \\ A &= \frac{\pi}{4} D^2 = \frac{\pi}{4} (0.025)^2 = 4.909 \times 10^{-4} \text{ m}^2 \\ h_f &= f \frac{L}{D} \frac{Q^2}{2gA^2} = f \frac{100}{0.025} \frac{Q^2}{2(9.81)(4.909 \times 10^{-4})^2} = 8.46 \times 10^8 f Q^2 \end{aligned}$$

The energy equation applied over 100 m of pipe is

$$\frac{p_1}{\gamma} + \frac{V^2}{2g} + z_1 = \frac{p_2}{\gamma} + \frac{V^2}{2g} + z_2 + h_f$$

which simplifies to

$$\begin{aligned} p_2 &= p_1 - \gamma(z_2 - z_1) - \gamma h_f \\ p_2 &= 550 - 9.79(100 \sin 10^\circ) - 9.79(8.46 \times 10^8 f Q^2) \\ p_2 &= 380.0 - 8.28 \times 10^9 f Q^2 \end{aligned}$$

(a) For $Q = 2 \text{ L/min} = 3.333 \times 10^{-5} \text{ m}^3/\text{s}$,

$$\begin{aligned} V &= \frac{Q}{A} = \frac{3.333 \times 10^{-5}}{4.909 \times 10^{-4}} = 0.06790 \text{ m/s} \\ \text{Re} &= \frac{VD}{\nu} = \frac{(0.06790)(0.025)}{1 \times 10^{-6}} = 1698 \end{aligned}$$

Since $\text{Re} < 2000$, the flow is laminar when $Q = 2 \text{ L/min}$. Hence,

$$\begin{aligned} f &= \frac{64}{\text{Re}} = \frac{64}{1698} = 0.03770 \\ p_2 &= 380.0 - 8.28 \times 10^9 (0.03770)(3.333 \times 10^{-5})^2 = 380 \text{ kPa} \end{aligned}$$

Therefore, when the flow is 2 L/min , the pressure at the downstream section is $\boxed{380 \text{ kPa}}$.
For $Q = 20 \text{ L/min} = 3.333 \times 10^{-4} \text{ m}^3/\text{s}$,

$$V = \frac{Q}{A} = \frac{3.333 \times 10^{-4}}{4.909 \times 10^{-4}} = 0.6790 \text{ m/s}$$